



STRUCTURAL
PORTFOLIO EDITION

CONSUCONSTRUCT

PORTFOLIO

—  BEST SELECTED WORK



Ing. David Chinchilla

STRUCTURAL ENGINEER · CICH 8222

EDUCATION

- CICH 8222**
Colegio de Ingenieros Civiles de Honduras — active license. Engineer of record; signs and stamps structural drawings.
- Ingeniero Civil**
Universidad Tecnológica Centroamericana (UNITEC), Honduras — degree in Civil Engineering.
- UNDP Registered Supplier**
Honduras — active registration for institutional / donor-funded works.

TOTAL EXPERIENCE

12+ years on site · 6 years as Founder / Technical Director

ABOUT ME

Licensed civil/structural engineer (CICH 8222), engineer of record on 14 design-calculation engagements out of 135 clients, with 12 years on site and 6 years running ConsuConstruct S. de R.L. in Honduras. Structural engines for ACI 318-19, AISC 360-16 LRFD §D-H+J and CHOC-08 Honduras seismic design coded from scratch — 22/22 unit tests — plus an ETABS 22 MCP bridge of my own for 3D modal / P-Delta verification. Chain delivered every time: soil study → ETABS model → signed design-calculation report → drawings with reinforcement → built work.

The same license runs the estimating and BIM pipeline behind every project I sign: EstimaStruct, a construction ERP I authored (1,145 partidas / 7,344 insumos / 367 unit-rate resources / 90 CSI chapters), and Brain, a LangGraph orchestrator (299-entry tool registry, cosine retrieval) — tools built to keep the site → Revit model → structural analysis → automated estimate → bank budget chain in one hand, not the other way around.

SOFTWARE EXPERIENCE

Structural codes ACI / AISC / CHOC-08	●●●●●●●●●●	22/22 unit tests
ETABS 22 (own MCP bridge)	●●●●●●●●●●	C# stdio connector
Revit — structural author / developer	●●●●●●●●●●	2,000+ hrs · 1,147 models
PostgreSQL + pgvector (estimating engine)	●●●●●●●●●●	68 tables · 7 schemas
FastAPI / Python (EstimaStruct backend)	●●●●●●●●●●	23 routers · 79 modules

VERIFIED TRACK RECORD

135

REAL CLIENTS — VERIFIED ENGAGEMENTS 2019–2026

6,317
FILES DELIVERED

26.34 GB
OF WORK DELIVERED

7 YEARS
OF CONTINUOUS OPERATION

24
WITH FORMAL CC CODE

99 closed / historical · 21 independent · 12 active now

SERVICE MIX

Administration / contracts / document support	100 cl · 74%
Renders / visualization / 3D assets	81 cl · 60%
Budgets / quantity take-off	72 cl · 53%
Drawings / technical documentation	69 cl · 51%
Site photos / site visits / progress	42 cl · 31%
BIM / Revit / 3D models	36 cl · 27%
Permits / municipal / bank	16 cl · 12%
Structural calculations / design reports	14 cl · 10%

NEW CLIENTS PER YEAR



LARGEST ENGAGEMENTS BY VOLUME

629	600 MB	commercial engagement CC-142	2026
586	182 MB	commercial engagement	2025
321	164 MB	extended commercial engagement	2021–2024
318	1.93 GB	single-family residence — structural calculation + BIM + budget CC130	2024–2026
303	4.37 GB	single-family residence — design + technical documentation CC-135	2024–2026
299	843 MB	independent professional engagement	2020–2023
284	1.00 GB	extended residential engagement	2019–2025
227	1.04 GB	Fittacori — structural calculation CC135	2024–2026
165	2.44 GB	residential engagement	2023–2025
153	727 MB	Gasolinera Puma Otoro	2023–2025

STRUCTURAL CALCULATION — 14 VERIFIED ENGAGEMENTS

01 single-family residence — structural calculation + BIM	CC130	02 Fittacori — structural calculation	CC135
03 single-family residence — structural calculation	CC-135	04 commercial project — structural calculation	CC137
05 structural calculation — private commission		06 design calculation report — independent client	
07 structural design — confidential commission		08 structural calculation — independent client	
09 design calculation report — private commission		10 structural design — independent client	
11 Foundation Drawings		12 structural calculation — confidential commission	
13 design calculation report — independent client		14 Proposals — design calculation report	CC-2024

SELECTED PROJECTS — VIEW LIVE

Villareal (https://consuconstruct.com/es/projects/villareal)	250 m²	Gasolinera Puma Otoro (https://consuconstruct.com/es/projects/gasolinera-puma-otoro)	6118 m²	Project Domes (https://consuconstruct.com/es/projects/project-domes)	200 m²
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TRACK RECORD — 12 YEARS

05 PROFESSIONAL CAREER (12 YEARS)

2013–2014	Purchasing and Administration — Constructora Briceño Vendor negotiation and administrative control. Learned the Honduran supply chain: cement, reinforcing steel, aggregates, formwork.
2014–2016	Resident Engineer's Assistant — Hyatt Hotel Large-scale hotel construction: reinforced concrete structure, MEP rough-in, finishes. RFIs, submittals, change orders.
2016–2018	General Services Coordinator — Corporación MCC Multi-site maintenance and minor capex. Quotes, purchase orders, supervision. Coordination across trades.
2019	Senior Estimator (remote) — Backmanage HN Quantity take-offs and budgets for U.S. general contractors. First contact with CSI MasterFormat and unit-price catalogs.
2020–present	Founder and Technical Director — ConsuConstruct S. de R.L. 135 clients end to end. Engineer of record: signs and seals structural drawings.

06 STRUCTURAL ENGINEERING

CICH 8222 — Colegio de Ingenieros Civiles de Honduras, active license. Engineer of record.
14 engagements with a design calculation report out of 135 clients.
Codes coded in-house in Python: **ACI 318-19 · AISC 360-16 LRFD §D-H + §J · CHOC-08 Honduras**
22/22 unit tests in the concrete engine.
ETABS 22 with a purpose-built MCP bridge in C# — 3D modal, P-Delta, seismic.

Chain delivered: soil study → ETABS model → signed design calculation report → drawings with reinforcement → built work.

01 TRACK RECORD (HEADLINE FIGURE)

135 real clients · 6,317 files delivered · 26.34 GB · 2019–2026
7 years of continuous operation · 24 with a formal CC code
99 closed · 21 independent · 12 active now

Verified against the client file index, not estimated.

03 GROWTH

New clients per year: 2019 · 2 — 2020 · 12 — 2021 · 12 — 2022 · 23 — 2023 · 21 — **2024 · 36** — 2025 · 13 — 2026 · 16

02 SERVICE MIX (ACROSS 135 CLIENTS)

SERVICE	CLIENTS	%
Administration / contracts / document support	100	74%
Renders / visualization / 3D assets	81	60%
Budgets / quantity take-off	72	53%
Drawings / technical documentation	69	51%
Site photos / site visits / progress	42	31%
BIM / Revit / 3D models	36	27%
Permits / municipal / bank	16	12%
Structural calculations / design reports	14	10%

01

**Reinforced Concrete ACI 318-19**

Design of beams, columns, slabs and footings under ACI 318-19.



02

**Structural Steel AISC 360-16 LRFD**

Steel members and connections: tension, compression, lateral-torsional buckling, bolts and welding.



03

**CHOC-08 Honduras Seismic Design**

Seismic classification, response spectra and storey drift control per CHOC-08.



04

**3D Analysis in ETABS**

3D modal modeling, P-Delta and seismic verification via a purpose-built MCP bridge in ETABS 22.

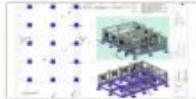
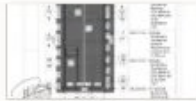
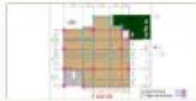
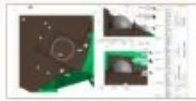
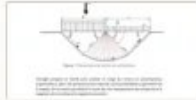


05

**Foundations**

Design of isolated footings, strip footings and foundation slabs.



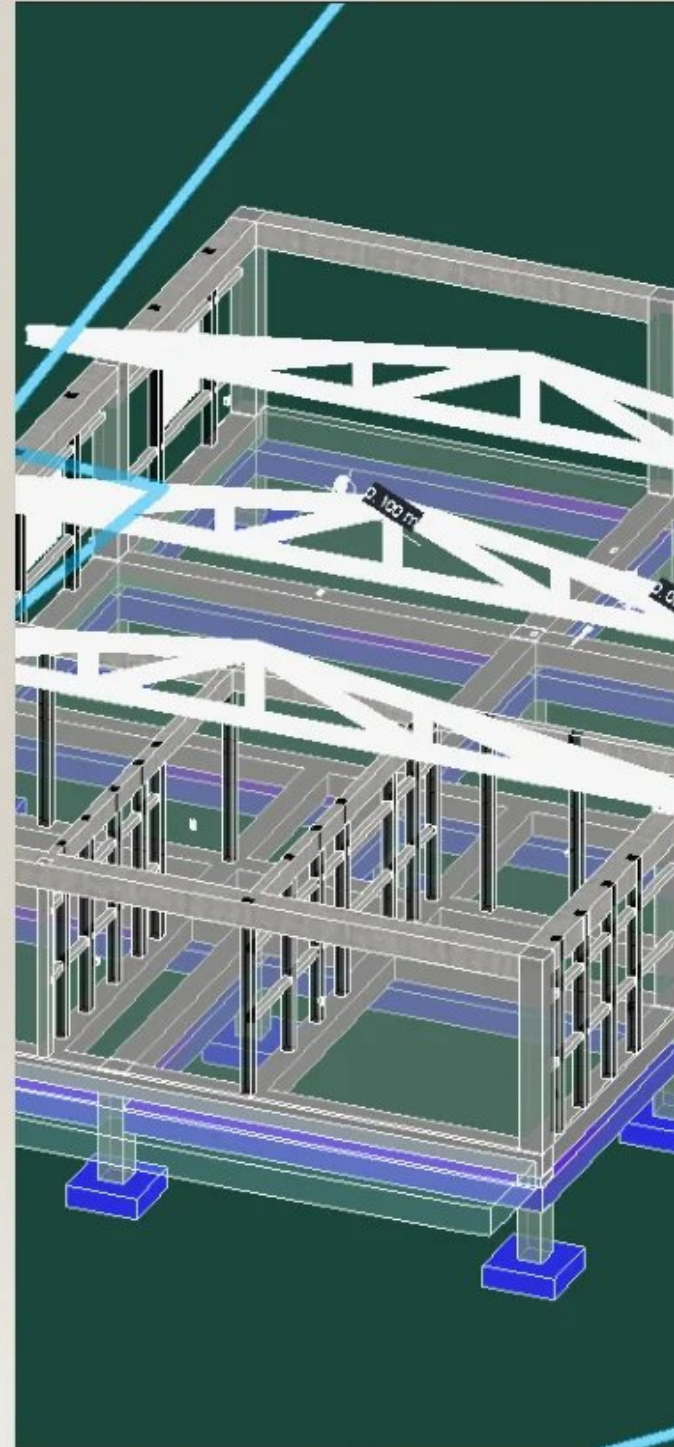
01		CC-135 FITTACORI TRAINING HALL PP. 2 — 11
02		CALCULATION ENGINE ACI 318-19 · AISC 360-16 LRFD · CHOC-08 PP. 14 — 14
03		GUIFARRO APARTMENTS SIGNED STRUCTURAL FRAMING PLAN — S-130 PP. 15 — 17
04		CC130 SINGLE-FAMILY RESIDENCE — STRUCTURAL CALCULATION PP. 18 — 19
05		OASIS RESIDENCE · FOUNDATION AND REINFORCEMENT PP. 20 — 21
06		VILLAREAL RESIDENCE · BIM STRUCTURAL FRAMING PLAN PP. 22 — 23
07		SINGLE-FAMILY RESIDENCE DESIGN AND TECHNICAL DOCUMENTATION — ETABS ANALYSIS PP. 24 — 25
08		RIVADENEIRA RESIDENCE · FOUNDATION — CC-223 PP. 26 — 27
09		TWO-LEVEL SINGLE-FAMILY RESIDENCE STRUCTURAL CALCULATION BY LEVEL — 2022 PP. 28 — 29
10		PROJECT DOMES PREFABRICATED DOMES — STRUCTURE AND FOUNDATIONS PP. 30 — 31
11		GASOLINERA PUMA OTORO GEOTECHNICAL STUDY — DESIGN BASIS PP. 32 — 33
12		COMMERCIAL AND OFFICE BUILDING REINFORCED CONCRETE STRUCTURE — MIXED USE PP. 34 — 35

CONTENT





BUILT VIEW



STRUCTURAL 3D MODEL

PROJECT DETAILS

Location: Estación Experimental Playitas, Cascabeles, Comayagua, Honduras.

Year: 2025–2026.

Designed by: Ing. David A. Chinchilla — CICH 8222.

Services:



PROJECT INTRODUCTION

Reinforced concrete frame with 8" reinforced masonry and a steel Pratt truss. Footprint 18.16×12.08 m, bays at 4.50 m. Z2 footings $1000 \times 1000 \times 300$ mm (9 #6 @ 0.10 o.c.), R5 columns 35×35 cm in 4000 psi concrete, S1 tie beam 35×35 , S2/S3 load-bearing bond beams 35×35 with 8 #6 and #4 stirrups @ 0.175. Roof structure in a 6-panel steel Pratt truss with L $3 \times 3 \times 1/4$ " angles, ASTM F1554 Gr. 36 $\varnothing 3/4$ " J-bolt anchors and E7018 fillet welding. Modeled and verified in ETABS 22 under ACI 318-19; maximum drift 1/969. Built and delivered.

STRUCTURAL DATA

STRUCTURAL SYSTEM	Concrete frame + 8" reinforced masonry + steel Pratt truss
CODES APPLIED	ACI 318-19 · CHOC-08 Honduras
MAX. STOREY DRIFT	1/969 (Comb1, X, L2 Bond-beam Level)
ANALYSIS	ETABS 22 — 3D model, purpose-built MCP bridge

CC-135 FITTACORI — DESIGN VS. DELIVERED

RENDER / BUILT — 1/2

RENDER

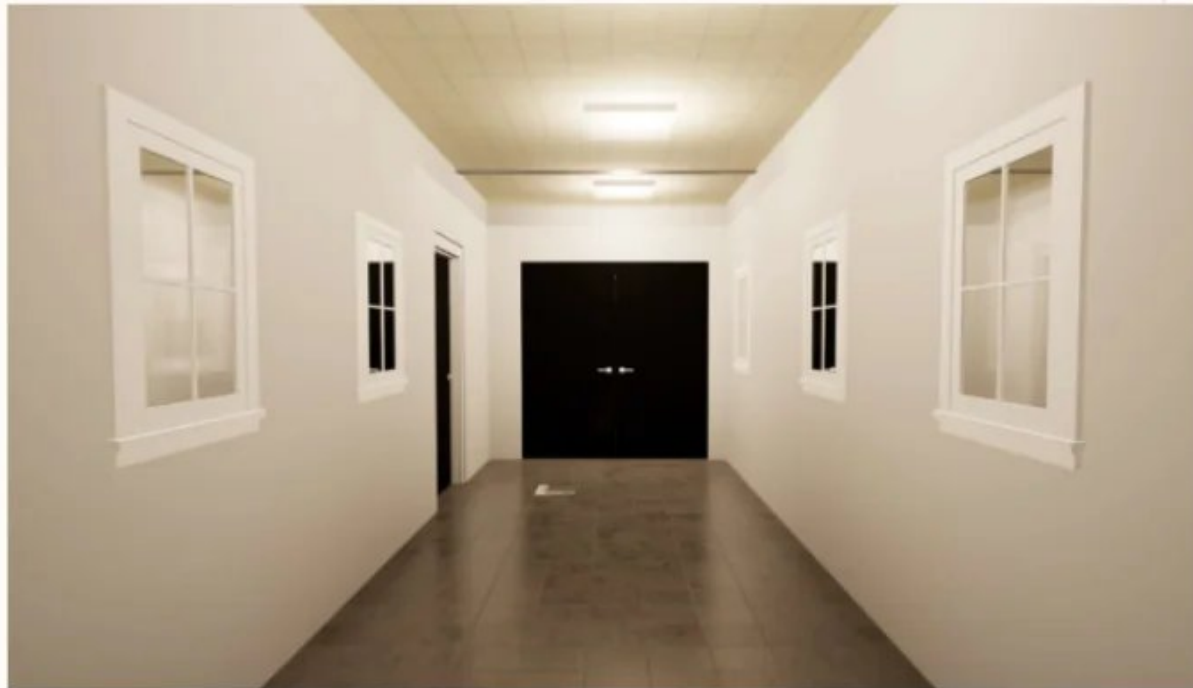
FACADE



BUILT



ENTRANCE HALLWAY



CC-135 FITTACORI — DESIGN VS. DELIVERED

RENDER / BUILT — 2/2

RENDER

BUILT

WORKSHOP CLASSROOM



EXTERIOR VIEW

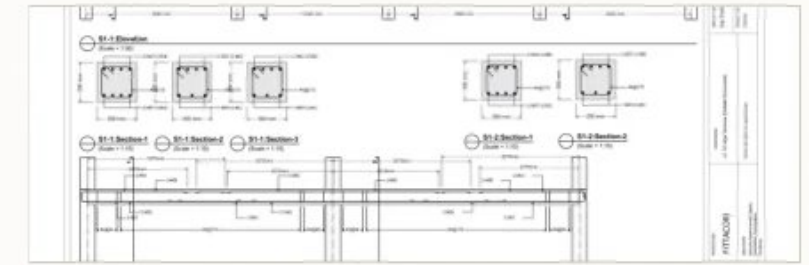


The structural frame spans a footprint of 18.16 × 12.08 m with bays at 4.50 m, in 4000 psi reinforced concrete. The Z2 footings, 1000×1000×300 mm (9 #6 @ 0.10 o.c.), transfer load to 35×35 cm R5 columns.

The S1 tie beam (35×35 cm) and the S2/S3 load-bearing bond beams (35×35 cm, 8 #6 and #4 stirrups @ 0.175) close the tie ring at the bond-beam level, the point where ETABS reports the model's most demanding storey drift.

The roof is resolved with a 6-panel steel Pratt truss in L 3×3×1/4" angles, bearing on the concrete frame and anchored with 3/4" diameter ASTM F1554 Gr. 36 J-bolts; the joints are fillet-welded with E7018 electrode.

The 3D model in ETABS 22 verifies the assembly under ACI 318-19 and CHOC-08: the maximum storey drift came out at 1/969, well below the code limit. The training hall is built and delivered.



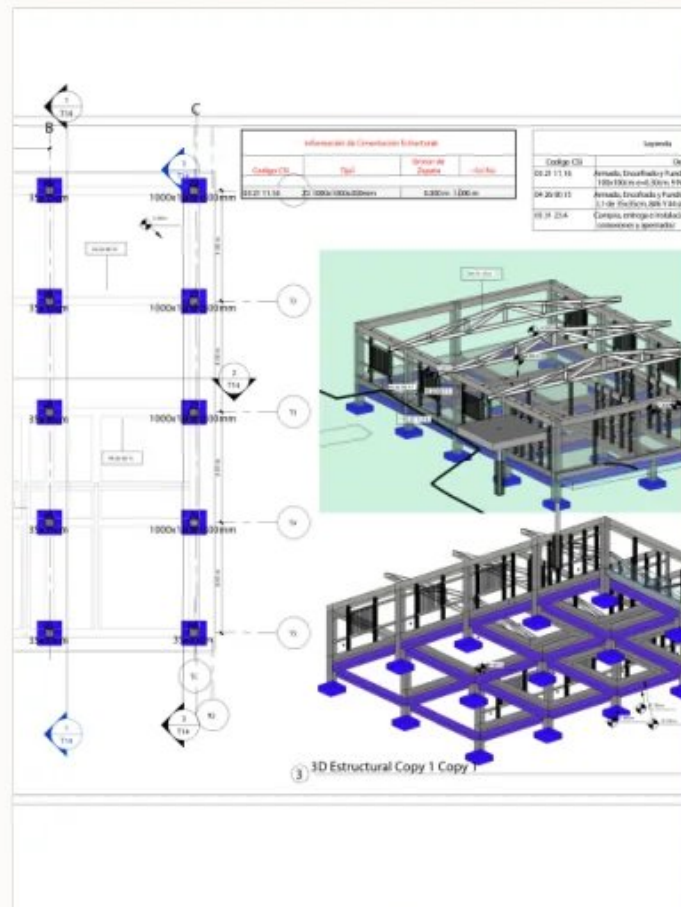
REBAR DETAIL



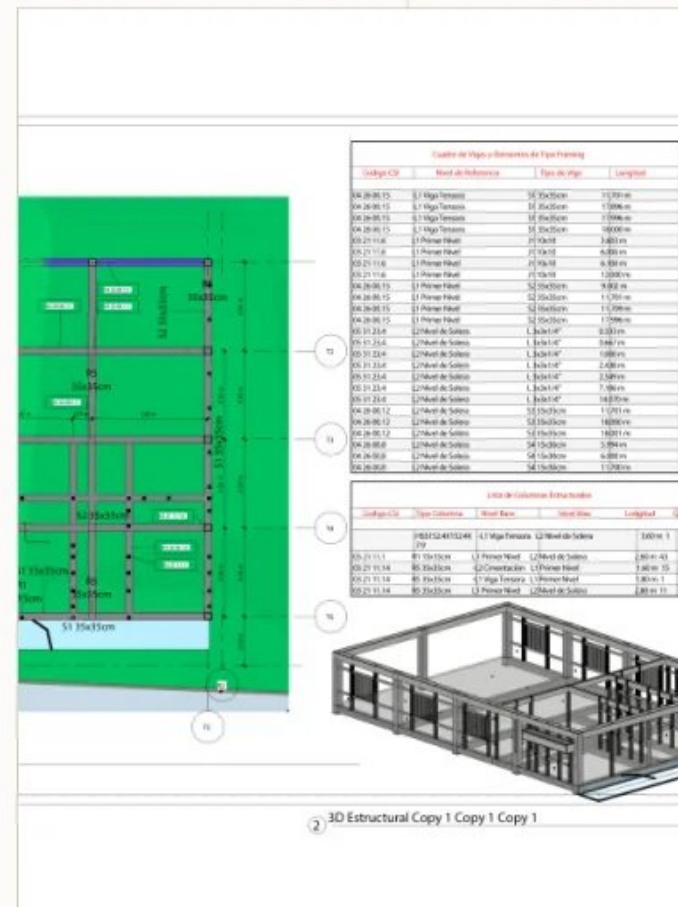
COLUMN DETAIL



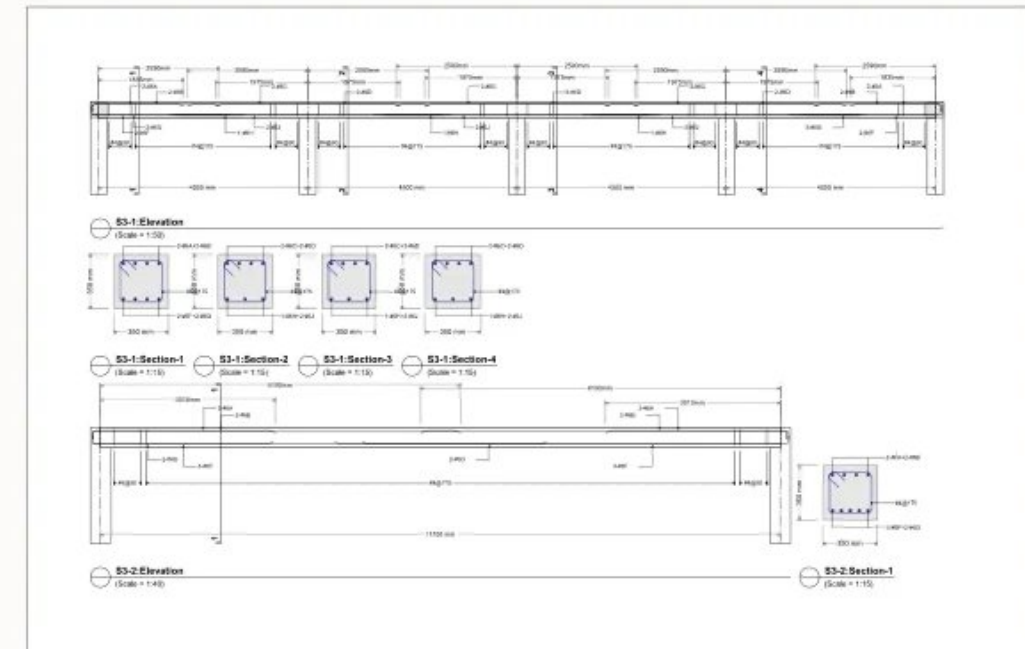
ROOF FRAMING – STEEL

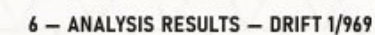


FOUNDATION PLAN



FRAMING PLAN





Modeled and verified in ETABS 22 under ACI 318-19. Maximum storey drift 1/969 (combination Comb1, X direction, bond-beam level), well below the code limit.

7 – ROOF FRAMING – STEEL
T06

8 – PRATT TRUSS DESIGN REPORT – AISC LRFD

9 — MOMENT ENVELOPE

10 - SOIL STUDY - QADM

11 — BEAM DRAWINGS — S-02 (39 SHEETS)

12 — COMPLETED WORK

TECHNICAL NOTE — ROOF

6-panel steel Pratt truss in L 3×3×1/4" angles, ASTM F1554 Gr. 36 Ø3/4" J-bolt anchors and E7018 fillet welding — design calculation report attached under the AISC LRFD code. Soil study and beam drawings (39 sheets, S-02) complete the technical file.

DESIGN CALCULATION REPORTS

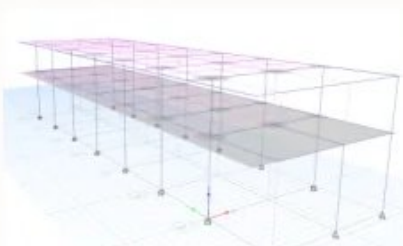
Soil study → ETABS model → signed design calculation report → drawings with reinforcement → built work. The complete chain, delivered and verifiable.

01 SOIL STUDY



CC-135 — LL, PI, AASHTO, Unified, ϕ , Cu, Q_{adm} , RQD, compaction. Basis of the foundation design.

02 ETABS MODEL



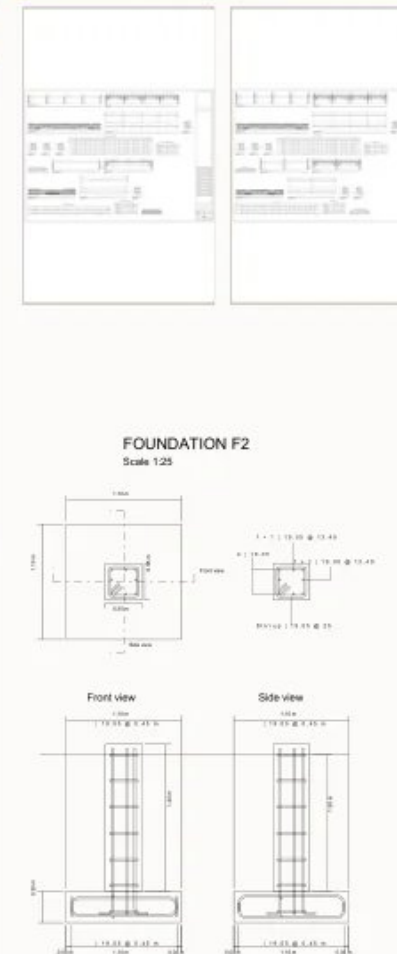
CC-135 — ETABS Model Explorer + 3D view of the structural model, verified under ACI 318-19.

03 SIGNED DESIGN REPORT



Two signed design reports, each 2+ pages — "By. Ing. David A. Chinchilla", letterheaded CONSUCONSTRUCT.COM.

04 DRAWINGS WITH REINFORCEMENT



CC-135 — a set of 39 sheets, elevations generated in ETABS with reinforcement.

05 COMPLETED WORK



CC-135 FITTACORI — delivered and in use. The chain closes in the field.

“No black boxes — every formula referenced to its clause, exportable straight to ETABS.”



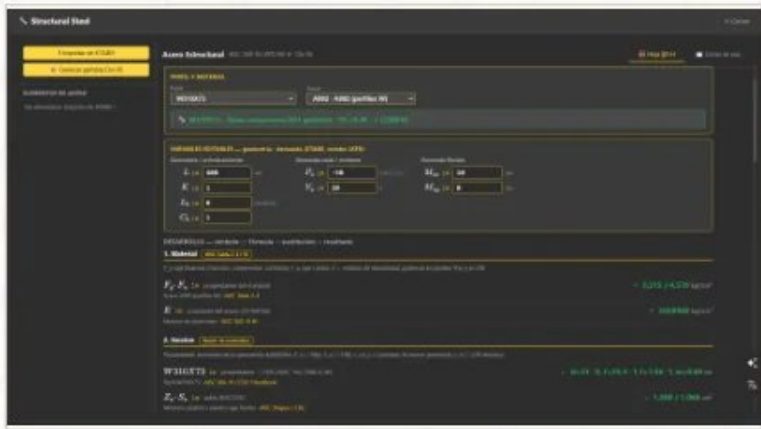
0 — LIVE CALCULATION SHEET — ACI 318-19

Element	Simple beam · 40 × 54 cm (d) · L = 6 m
Materials	$f'c = 210 \cdot f'y = 4,200 \text{ kg/cm}^2$
Demand	$Mu = 52 \text{ t}\cdot\text{m} \cdot Vu = 21 \text{ t}$
β_1	0.85 for $f'c \leq 280 \cdot \text{ACI 318-19 §22.2.2.3}$
Ec	$15100 \cdot \sqrt{f'c} = 218,820 \text{ kg/cm}^2 \cdot §19.2.2.1$
pb	0.02125 — seismic cap $pmax = 0.5 \cdot pb$
Factors	$\phi flex 0.90 \cdot \phi shear 0.75 \cdot \phi col 0.65$



1 — SEISMIC SPECTRUM — CHOC-08

Seismic zone	3b (Z = 0.25)
S1 · I · Rw	$S1 \cdot I = 1 \cdot Rw = 8$
Period T	0.167 s
Coefficient C	2.75
Base shear V	104 kgf
Ordinate a/g(T)	$0.6875 \text{ g} \cdot \text{CHOC-08 §1.3.6}$
Drift limit	$\min(0.04/Rw, 0.005) = 0.005 \cdot \text{CHOC-08 §1.3.5.8.2}$



2 — STEEL DESIGN — AISC 360-16 LRFD §D-H

Section	W310X73
Steel	A992
Governing equation	Combined flexure-compression §H1
D/C	0.48
Result	PASSES

CRITERION

A calculation you cannot audit is not a calculation, it is an assertion. Every formula in the engine prints its exact clause — ACI 318-19, AISC 360-16, CHOC-08 — with the numeric substitution in plain view, so an outside reviewer can follow the reasoning line by line without opening the code. The CHOC-08 spectrum exports straight into the ETABS model: the same code that gets calculated is the one that gets run.

**S-130 — STRUCTURAL FRAMING PLAN N-1**

CLIENT: ANGELA GUIFARRO · DRAWN AND SIGNED BY ING. DAVID CHINCHILLA, CICH SEAL. LEGEND:

03 31 13.2

CONCRETO PREMEZCLADO 3,000 PSI, AGREGADO 3/4", REVENIMIENTO 6 ·

04 21 11.2

COLUMNA 60×60, 8N°6, N°3@30 ·

04 21 11.10

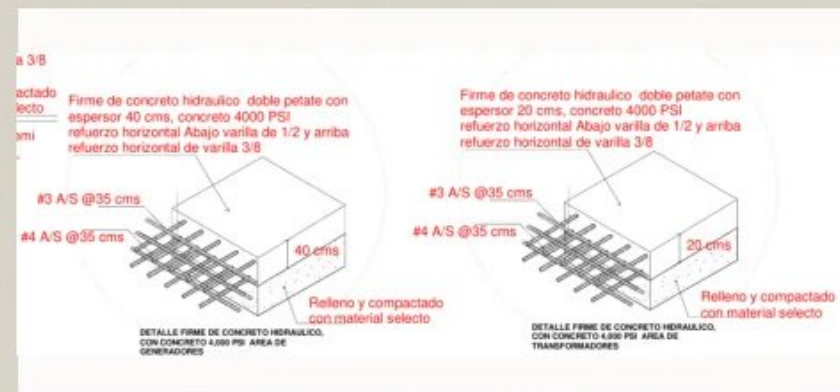
ZAPATA AISLADA 2.00×2.00, E=0.37, 11 N°6 A.S. ·

04 21 11.13

COLUMNA 40×40, 6N°6, N°3@20 ·

04 26 00.10

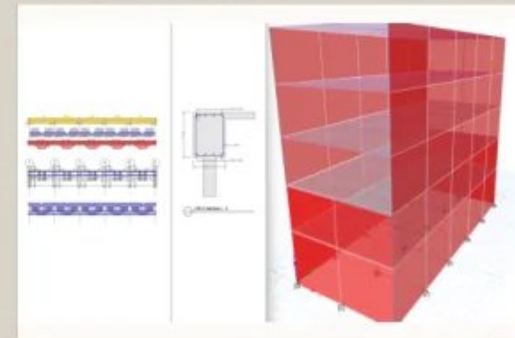
V-1 VIGA B=30 H=40, 4#5, #3@30, CONCRETO 1:2:2.

**A106 — DIMENSIONED PLAN, PARKING**

TRAPICHE APARTMENTS PROJECT, EL TRAPICHE · FOUNDATION SLAB WITH 6"×6" 4/4 WELDED WIRE MESH, 8 M' UNDERGROUND CISTERN. SIGNED, CICH 8222.

**S-131 — STRUCTURAL FRAMING PLAN LEVEL 1**

SAME CSI KEYNOTE SET, ONE LEVEL UP.

**ETABS MODEL AND CB-1 SECTION**

3D MODEL OF THE 4 LEVELS WITH SLABS AND SUPPORTS · JOIST-AND-BLOCK DETAILING BY GRIDLINE · CB-1 BEAM SECTION 50 × 70 CM WITH 2-#5 A + 2-#5 B TOP, 2-#5 F + 2-#5 G BOTTOM AND 3-#11.

PROJECT DETAILS**Period:** 2024 (Jan–Oct).**Role:** Engineer of record · Structural design — CICH 8222.**SCOPE**

Structural framing plan N-1 (sheet S-130) signed and sealed by CICH, with a complete keynote legend — isolated footings 2.00×2.00 m, columns 60×60 and 40×40 cm, beam V-1 30×40 cm, 3,000 psi ready-mix concrete. Accompanied by the level-1 framing plan (S-131) and the dimensioned parking plan (A106, Trapiche Apartments Project, El Trapiche) — the most complete structural drawing set in the archive outside CC-135 Fittacori.

View project on consuconstruct.com → (<https://consuconstruct.com/es/projects/guifarro-apartments>)

**GUIFARRO
APARTMENTS****SIGNED STRUCTURAL FRAMING
PLAN — S-130**

3



aproximadamente a **5.05 metros de profundidad**. Esto es porque se identificó un estrato con una capacidad soportante de 3.0 kg/cm², la cual es más adecuada para soportar la carga de un complejo de apartamentos de 4 niveles. Además, se debe garantizar que el estrato donde se fundará está alejado del talud natural por al menos 2 metros de la cara de la cimentación.



El complejo de apartamentos está compuesto por cuatro niveles, con una altura total aproximada de 13.6 metros. El diseño estructural incorpora columnas de concreto armado, vigas y losas, junto con zapatas de cimentación, todos especificados para soportar las cargas previstas y garantizar una distribución eficiente de las mismas. La estructura está diseñada para responder a las necesidades funcionales y estéticas del proyecto, a la vez que se adapta a las características del terreno.

Características Estructurales

Columnas:
Dimensiones: 60x60 cm.
Función: Las columnas están diseñadas para transferir las cargas verticales de los pisos y la estructura a las zapatas, garantizando la estabilidad del edificio.

Vigas:
Dimensiones: 30x40 cm.
Función: Las vigas están diseñadas para soportar las cargas de las losas y transferirlas a las columnas y zapatas, proporcionando rigidez lateral y resistencia a la flexión.

Losas:
Espesor: 15 cm.
Función: Las losas distribuyen las cargas de uso y muertas de manera uniforme a las vigas, contribuyendo a la estabilidad del edificio y a la creación de espacios habitables.

Zapatas:
Dimensiones: 2x2 m con un espesor de 40 cm.
Función: Las zapatas están diseñadas para distribuir las cargas de las columnas al suelo, adaptándose a las características del terreno según el estudio geotécnico.

Integración con el Diseño Arquitectónico
El diseño estructural complementa al diseño arquitectónico del complejo, al garantizar que los elementos estructurales no solo cumplen con los requisitos técnicos, y de

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1 — DESCRIPTIVE REPORT

5



igualmente según las condiciones de curado y la composición específica del concreto.

2. Resistencia a la Compresión (f_c):

- La resistencia característica a la compresión es **3000 psi (20.7 MPa)**, lo que representa la capacidad del concreto para soportar cargas de compresión antes de fallar.

3. Módulo de Poisson (ν):

- El módulo de Poisson para el concreto se sitúa generalmente alrededor de **0.2 a 0.3**, representando la relación entre la deformación lateral y la deformación longitudinal bajo carga.

Estas propiedades son críticas para la determinación de las deformaciones, la capacidad de carga, y la estabilidad estructural del edificio en los cálculos de diseño, asegurando que el concreto utilizado cumpla con los requisitos de resistencia y comportamiento esperados para el proyecto.

Cargas consideradas:

- Carga Muerta (DL):** Peso de losas, vigas, columnas, y zapatas.
- Carga Viva (LL):** 750 kg/m².
- Cargas de Viento (W):** A determinar según normativas CHOC - 08.
- Cargas Sísmicas (S):** A determinar según normativa.

2. Propiedades de los Elementos Estructurales

- Columnas:**
 - Sección: 60x60 cm.
 - Refuerzo: 8 varillas de 6/8".
 - Estribos: #3" a cada 30 cm.
 - Concreto: f_c = 250 kg/cm².
 - Densidad del concreto: 2400 kg/m³.
- Vigas:**
 - Sección: 30x40 cm.
 - Refuerzo: 4 varillas de 5/8".
 - Estribos: #3" a cada 30 cm.
 - Concreto: f_c = 250 kg/cm².
 - Densidad del concreto: 2400 kg/m³.
- Losas:**
 - Espesor: 15 cm.
 - Refuerzo: Varillas de refuerzo según el diseño estructural.
 - Concreto: f_c = 250 kg/cm².
 - Densidad del concreto: 2400 kg/m³.

consuconstruct.com

2 — ELEMENT SPECIFICATIONS

INFORME GEOTÉCNICO, ENSAYO DE PENETRACIÓN ESTÁTICA
PROYECTO: VIVIENDA GUIFARRO MEDIO

7. Conclusiones y recomendaciones

a) El material encontrado en el primer metro presenta valores de capacidad soportante de 1.8 a 1.9 kg/cm² debido a que la arcilla estaba en una condición seca, sin embargo en presencia de agua estos materiales pierden gran parte de su capacidad soportante.

b) De los 0.95 a los 2.25 el material presenta una baja capacidad soportante, con valores de 0.8 a 1.0 kg/cm², consistente con materiales en compactación.

c) Durante el ensayo en el tramo de 2.25 a 2.70 encontró un tramo resistente con una capacidad soportante de 2.6 kg/cm², este pudo deberse a una pozo de mediana tamaño, como las observadas en las excavaciones contiguas al sitio donde se realizó el ensayo.

d) En el tramo de 2.70 a 3.60 en ensayo reportó una capacidad de 1.60 debido a que la arcilla estaba en una condición seca, sin embargo, en presencia de agua estos materiales pierden gran parte de su capacidad soportante.

e) El material encontrado es una arena arcillosa, la cual es susceptible a cambios en su capacidad soportante en presencia del agua, por lo que se recomienda no cimentar en estos materiales ya que son aptos para soportar carga debido a su baja compactación.

f) El ensayo se detuvo a 4.05 debido a que el equipo reportó rechazo, reportando una capacidad soportante de 3 kg/cm², por lo que se recomienda profundizar por lo menos a un metro por debajo de este nivel y verificar que dicho estrato esté alejado del talud natural por lo menos 2 metros de la cara de la cimentación.

12

3 — SPT TEST — BORINGS
CINSA · MAR 2024

INFORME GEOTÉCNICO, ENSAYO DE PENETRACIÓN ESTÁTICA
PROYECTO: VIVIENDA GUIFARRO MEDIO

6. Resultados Obtenidos

6.1 Análisis de Capacidad Soportante del suelo

El análisis de capacidad soportante del suelo se realizó en base al número de golpes (N) obtenidos del ensayo de penetración estándar realizado en campo y los resultados de los ensayos de clasificación de las muestras alteradas obtenidas del SPT.

Para el cálculo de la capacidad soportante del suelo se consideró una zapata de denominación unitaria y un factor de seguridad de 4.

Para suelos granulares se utilizó el factor de corrección CN (según el factor de corrección recomendado por Lee - Whitman (1986)) y es el siguiente:

$$CN = 9.78 \left[\frac{1}{N_{60}} \right]^{1.5} \text{ en } \text{Kg/cm}^2$$

los resultados se muestran en la tabla que se presentan a continuación:

Tabla 1. Variación del coeficiente η_{fs}

1. Variación de η_{fs}			
País	Tipo de martinete	Ubicación del martinete	η_{fs} (%)
Japón	Tonido	Cuadrado libre	78
	Tonido	Cuadrado y polo	67
Estados Unidos	De seguridad	Cuadrado y polo	60
	Tonido	Cuadrado y polo	45
Argentina	Tonido	Cuadrado y polo	45
China	Tonido	Cuadrado libre	60
	Tonido	Cuadrado y polo	50

9

4 — BEARING CAPACITY BY STRATUM

INFORME GEOTÉCNICO, ENSAYO DE PENETRACIÓN ESTÁTICA
PROYECTO: VIVIENDA GUIFARRO MEDIO

Tabla de resultados de Capacidad Soportante del Suelo, Sonda No. 1.

Profundidad (m)	Tipo de Suelo	N ₆₀	f _u	Q (kg/m ²)	f _s (kg/cm ²)	Descripción
0.00 - 0.45	CL	23	33	-	1.9	Materiales arcillosos, medianos finos, y arcillas plásticas de consistencia blanda y líquida, color rojo de baja compactabilidad.
0.45 - 0.90	SC	21	30	-	1.8	
0.90 - 1.35	SC	7	29	-	0.8	
1.35 - 1.80	SC	9	30	-	1.0	
1.80 - 2.25	SC	8	29	-	0.9	Materiales arena arcillosos, suelos con presencia de grava, arena fina y arcillas plásticas de consistencia blanda y líquida, color rojo.
2.25 - 2.75	SC	36	35	-	2.6	
2.70 - 3.15	SC	18	31	-	1.6	
3.15 - 3.60	SC	17	31	-	1.6	
3.60 - 4.05	SC	43	34	-	3.0	

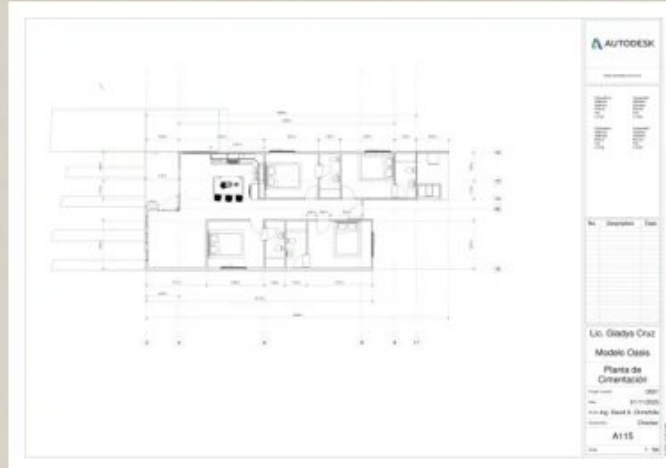
Nota:
En los ensayos no se detectó la presencia del nivel freático. En suelos cohesionados el valor de la cohesión se calculó solamente como referencia, en base a correlaciones proporcionadas por Terzaghi & Peck. El valor de capacidad de carga admisible se calculó sin tener en cuenta el componente de la cohesión, y se analizó empleando únicamente el ángulo de fricción interna del material. La capacidad de carga admisible se calculó mediante la fórmula de Terzaghi, para una zapata cuadrada de 1 m x 1 m, y aplicando un Factor de Seguridad de 3.

11

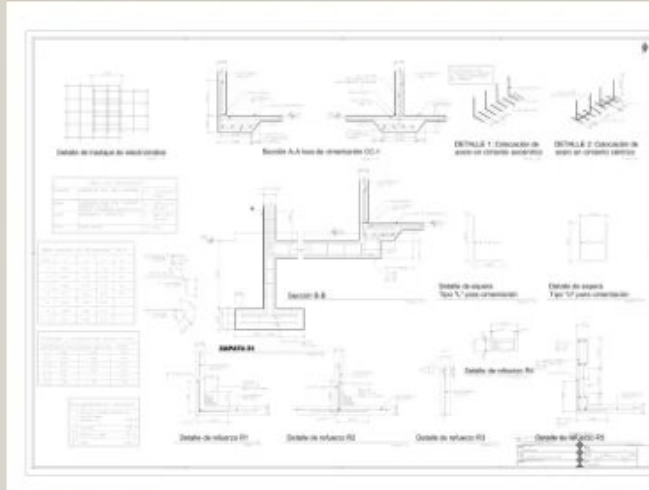
5 — STRATIGRAPHIC PROFILE

TECHNICAL NOTE

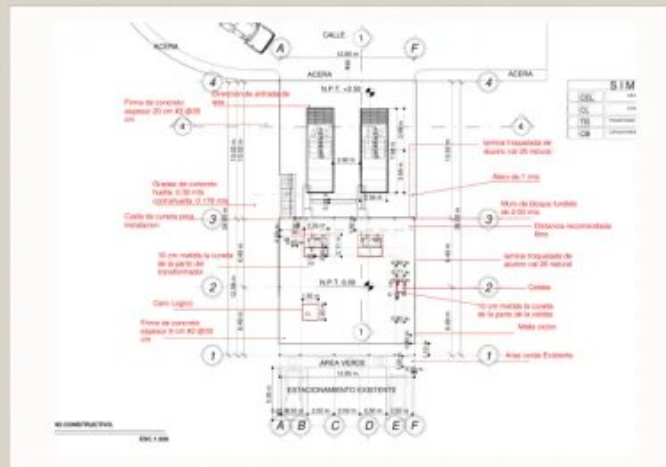
4-level apartment complex, total height 13.6 m. The site's Standard Penetration Test (SPT) — commissioned by the owner — delivers the bearing capacity by stratum: clayey and sandy-clay materials, CL and SC type, N₆₀ between 7 and 43, internal friction angle 29°–35°, and q_a calculated by the Terzaghi formula for a 1×1 m square footing with a safety factor of 3. Foundation is set on that data at ≈5.05 m, targeting the 3.0 kg/cm² stratum. Columns 60×60 cm (8 no. 6/8" bars) and 40×40 cm (6 no. 6/8" bars), beams 30×40 cm (4 no. 5/8" bars), slabs e=15 cm, f_c=250 kg/cm², live load 750 kg/m² under CHOC-08. Soil first, section second.



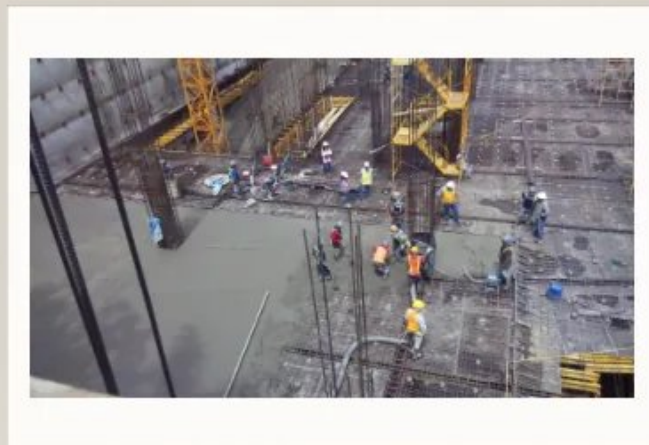
A115 — FOUNDATION PLAN
SCALE 1:20.



A114 — SLAB REINFORCEMENT DETAIL
REINFORCED FOUNDATION.



A118 — BLOCK REINFORCEMENT
VERTICAL REINFORCEMENT DETAIL.



DOWEL DETAILS
CCL / CCU.

PROJECT DETAILS

Period: 2020–2023.

Area: 120 m².

Role: Engineer of record · Structural design — CICH 8222.

SCOPE

Reinforced masonry resolved down to the detail, which is where Honduran construction fails. Vertical reinforcement housed inside the block cells, filled with 1:3:5 flowable concrete poured in lifts as the wall rises. Horizontal reinforcement in the mortar joints: 1#3 above the first course, 1#3 every 3 courses (≈ 0.63 m) and 1#3 below the top course. Minimum 30 cm horizontal reinforcement at every wall intersection. 1:4 mortar, type 1 cement with washed river sand. Common bond beam 15 cm with 4#3 and closed tie. Details of filled 6" block for corners and intersections at 1:10 and 1:20 scale, plus a reinforced foundation slab and CCL/CCU column dowels. Three years of documented work.

View project on [consuconstruct.com](https://consuconstruct.com/es/projects/oasis) → (<https://consuconstruct.com/es/projects/oasis>)



OASIS

RESIDENCE · FOUNDATION AND
REINFORCEMENT

**A133 — 2ND LEVEL STRUCTURAL FRAMING PLAN**

OWNER: BENJAMÍN VARELA · LOARQUE, TEGUCIGALPA, F.M. · DESIGNED, REVIEWED AND APPROVED BY: ING. DAVID CHINCHILLA CICH 8222 · SCALE 1:75 · 2024-07-17. LEGEND BY COLOR — PRIMARY BEAM

72.9 M

· SECONDARY BEAMS

70.3 M

· PRIMARY COLUMNS

11

· CONFINING COLUMNS

17

·

PROJECT DETAILS**Location:** Loarque, Tegucigalpa, F.M.**Period:** 2023–2025.**Area:** 250 m².**Role:** Engineer of record · Structural design — CICH 8222.**SCOPE**

Foundation slab with welded wire mesh, Z1 footings 80×80 cm, strip footing 0.40×0.20 m, R2 columns and V-2 beams 0.15×0.30 m, joist-and-block floor system. The deepest BIM project in the archive: 64 in-house Revit models, including dedicated plumbing and electrical, coordinated via MCP.

View project on consuconstruct.com → (<https://consuconstruct.com/es/projects/villareal>)

**VILLAREAL**

RESIDENCE · BIM STRUCTURAL
FRAMING PLAN

PROJECT DETAILS**Period:** 2024–2026.**Role:** Engineer of record · Structural design — CICH 8222.**SCOPE**

ETABS analysis resolved level by level, with an independent structural framing plan for L1 and L2 instead of a single repeated plan. Three model reports and a technical annex complete the verification file.

**ETABS 3D MODEL — TWO LEVELS**

ETABS REPORT COVER WITH ISOMETRIC VIEW OF THE STRUCTURAL MODEL, BASIS OF 3 ETABS REPORTS + TECHNICAL ANNEX (L1/L2).

**SINGLE-FAMILY RESIDENCE**

DESIGN AND TECHNICAL
DOCUMENTATION — ETABS
ANALYSIS

PROJECT DETAILS

Period: 2023–2025.

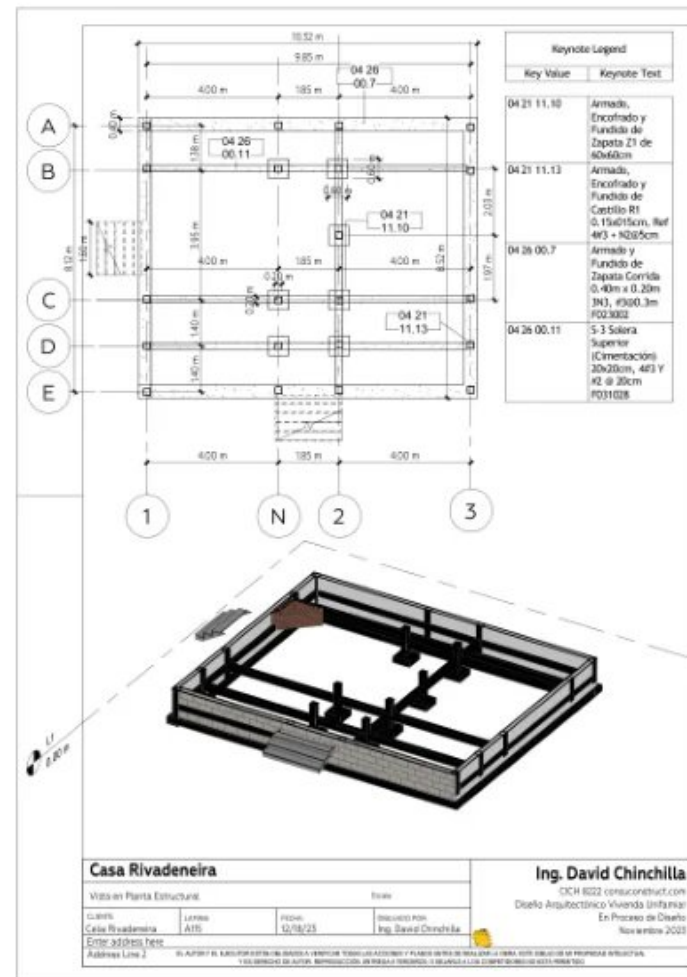
Area: 50 m².

Role: Engineer of record · Structural design — CICH 8222.

SCOPE

Z1 footing 60×60 cm and R1 confining column 0.15×0.15 m reinforced with 4#3 plus N2 @ 5 cm. Strip footing 0.40×0.20 m with 3N3 and #3 @ 0.30 m, capped by an S-3 upper foundation bond beam 20×20 cm with 4#3 and #2 @ 20 cm. Complete foundation on a 50 m² house: small section, full engineering rigor. Built and photographed.

View project on consuconstruct.com → (<https://consuconstruct.com/es/projects/rivadeneira>)



A115 — STRUCTURAL FOUNDATION

RIVADENEIRA

RESIDENCE · FOUNDATION — CC-223

PROJECT DETAILS

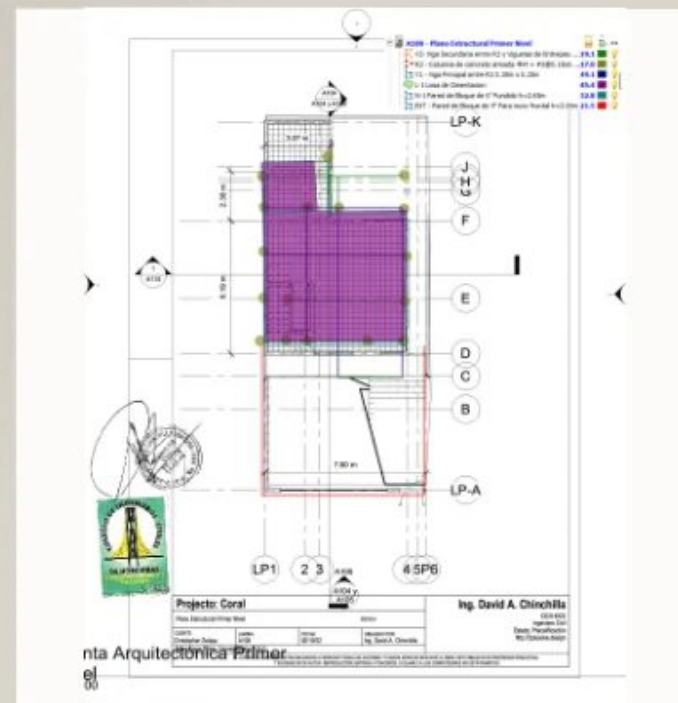
Period: 2022–2023.

Role: Engineer of record · Structural design — CICH 8222.

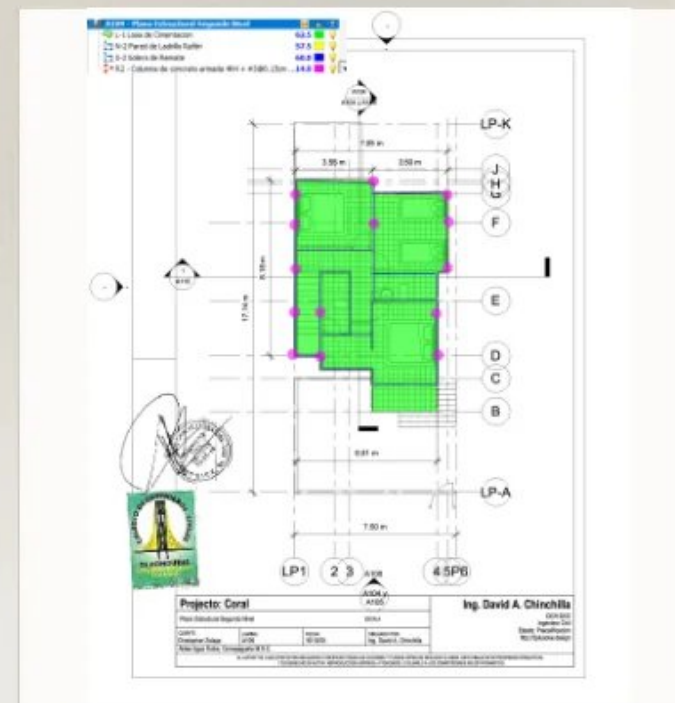
SCOPE

Complete structural drawing set resolved level by level: foundation, first-level framing and second-level framing, each on its own sheet. The set closes with a combined structural-and-electrical drawing — coordination between disciplines drawn on the same sheet, not resolved in the field.

A108 — FOUNDATION PLAN



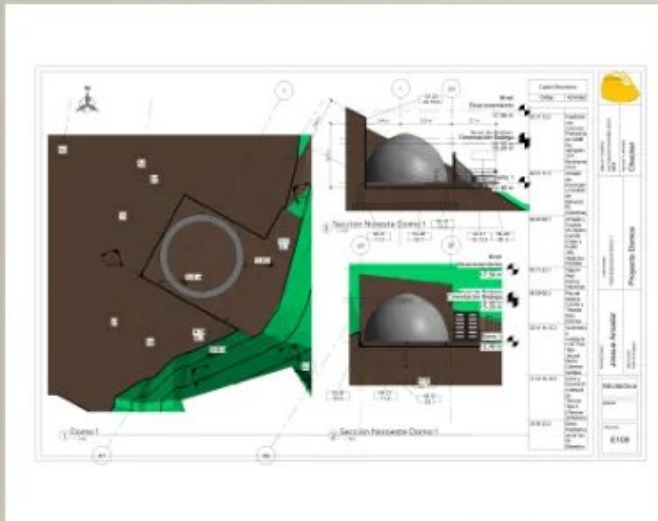
A108 — FIRST-LEVEL FRAMING



A109 — SECOND-LEVEL FRAMING

TWO-LEVEL SINGLE-FAMILY RESIDENCE

STRUCTURAL CALCULATION BY LEVEL — 2022

**E109 — STRUCTURAL VIEW, DOME 1**

OWNER: JOSSE AMADOR · DRAWING AND DESIGN BY ING. DAVID CHINCHILLA, CICH 8222. SCHEDULE:

04 26 00.7

ZAPATA CORRIDA 0.40×0.20, 3N3, #3@0.3M ·

05 31 23.1

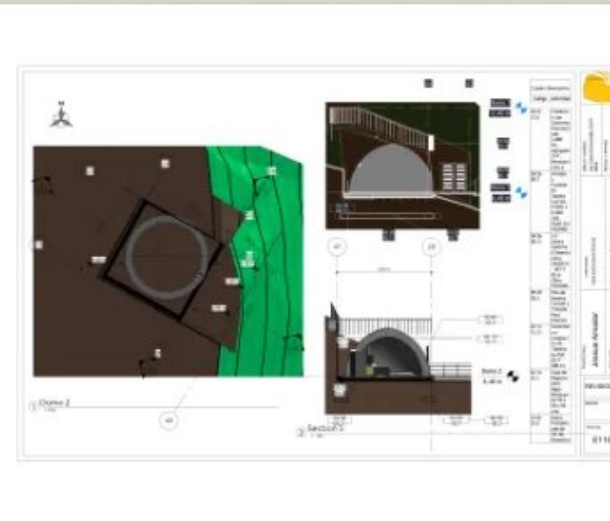
VIGAS H PARA PÓRICO INDUSTRIAL ·

33 92 23.2

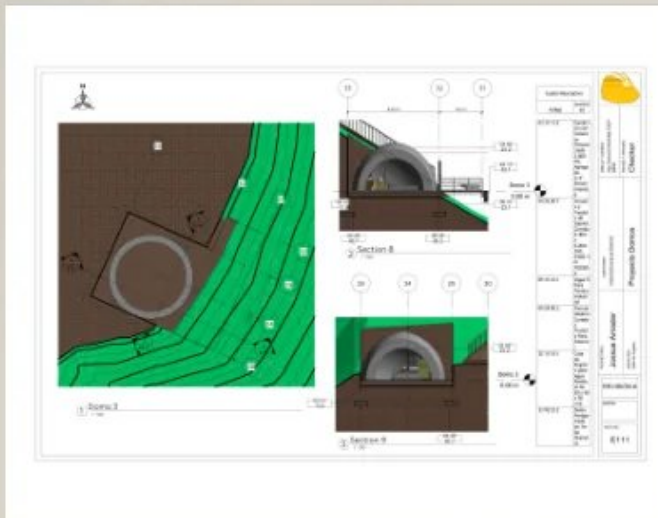
DOMO PREFABRICADO DE 3 M DE DIÁMETRO ·

31 23 16.6

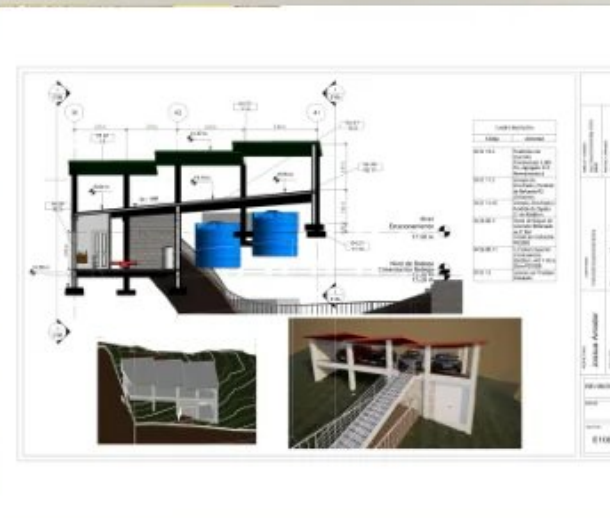
CORTE Y EXCAVACIÓN EN TERRENO SEMIDURO. LEVELS 17.50 / 15.20 / 12.40 M.

**E110 — STRUCTURAL VIEW, DOME 2**

S-3 STRIP FOOTING 20×20 CM — SAME CSI SCHEDULE, NORTHEAST/NORTHWEST SECTIONS.

**E111 — STRUCTURAL VIEW, DOME 3**

SECTIONS 8/9 — CUT AND EXCAVATION IN SEMI-HARD GROUND, STRIP-FOOTING FOUNDATION.

**E105 — PARKING, EAST VIEW**

STEEL INDUSTRIAL PORTAL FRAME AND WATER CISTERNS — SAME STRUCTURAL ASSEMBLY, WITH A 3D RENDER OF THE ROOF AND ACCESS POINTS.

PROJECT DETAILS

Location: Valle de Ángeles, Honduras.

Period: 2024.

Area: 200 m².

Role: Engineer of record · Structural design — CICH 8222.

SCOPE

Three prefabricated 3 m diameter domes on industrial steel portal frames (H-beams) and strip footings, plus an adjacent parking structure. Platform levels 17.50 / 15.50 / 12.40 m, with cut and excavation in semi-hard ground. Sheet E109 signed — **drawing and design: Ing. David Chinchilla, CICH 8222.**

View project on consuconstruct.com → (<https://consuconstruct.com/es/projects/project-domes>)

PROJECT DOMES

PREFABRICATED DOMES —
STRUCTURE AND FOUNDATIONS



GASOLINERA PUMA OTORO

GEOTECHNICAL STUDY — DESIGN BASIS

INFORME GEOTÉCNICO

6 INGENIERÍA EN CIMENTACIONES

6.1 Capacidad Portante

Se denomina como capacidad de carga admisible de una cimentación aquella carga que al ser aplicada no provoque falla o daños en la estructura soportada. La capacidad de carga no solo está en función de las características del suelo, sino que depende del tipo de cimentación y del factor de seguridad adoptado. En la Fig.7 se observa el mecanismo de rotura en una cimentación poco profunda.

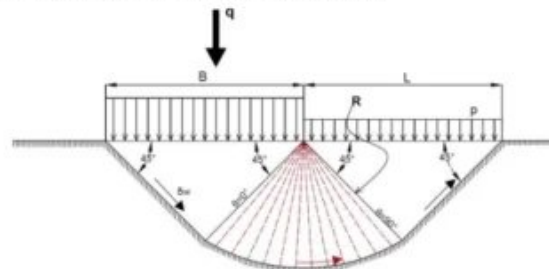


Figura. 7 Mecanismo de rotura en cimentación.

Terzaghi propuso la teoría para evaluar la carga de rotura en cimentaciones superficiales a partir de parámetros del material, de la profundidad y geometría de la zapata, de tal modo que definió la suma de tres componentes que determinan la carga de rotura mediante la siguiente ecuación:

$$q_u = 1.3cN_c + qN_q + 0.4\gamma BN_\gamma$$

Donde:

C = Cohesión del suelo

γ = Peso unitario del suelo

q = γD_f

N_c, N_q, N_γ = factores de capacidad de carga adimensionales

ESTUDIO DE SUELOS — GASOLINERA PUMA — JESÚS DE OTORO

24

BEARING CAPACITY — TERZAGHI'S THEORY

FAILURE MECHANISM IN A SHALLOW FOUNDATION, ULTIMATE BEARING EQUATION Q_u .

Por lo cual, el valor de capacidad de carga admisible se presenta en la siguiente ecuación:

$$q_{adm} = \frac{q_u}{F.S.}$$

La Tabla.7 muestra los resultados de la capacidad admisible calculada utilizando la ecuación correspondiente, usando un factor de seguridad de 3.50. Es importante destacar que la capacidad admisible es un parámetro fundamental en el diseño geotécnico de las cimentaciones, ya que representa la máxima carga que puede soportar el suelo sin experimentar falla o deformación excesiva.

Tabla.6 Parámetros de diseño

Descripción	Unidad	Valor
Peso específico (γ)	(kN/m ³)	20.11
Cohesión (C_u)	(kN/m ²)	0
Ángulo de fricción	(°)	18
Factor de seguridad	(-)	3.5
Cimentación cuadrada	(m)	1.2
Profundidad de empalme	(m)	1.0
Nivel freático		No se observó

Tabla.7 Capacidad admisible

Capacidad admisible (q_{adm})	Unidad	Valor
	kgf/cm ²	1.82
	tonf/m ²	20.11

El ingeniero encargado del diseño de la cimentación y retención será responsable de seleccionar el tipo adecuado de estructuras y los niveles de empalme que satisfagan las necesidades de las estructuras en conjunto con la información proporcionada en este informe. Debe considerar los factores relevantes como las cargas a soportar, las características del suelo/roca, los materiales disponibles y otros factores que pueden afectar la estabilidad y seguridad de la estructura. Además, el ingeniero debe cumplir con los requisitos de las normativas y regulaciones locales para garantizar la construcción segura y sostenible de la obra.

ESTUDIO DE SUELOS — GASOLINERA PUMA — JESÚS DE OTORO

25

DESIGN PARAMETERS AND ALLOWABLE CAPACITY

$Q_{ADM} = 1.82 \text{ KGF/CM}^2 \text{ (20.11 TONF/M}^2\text{)} \cdot FS=3.5 \cdot 1.2 \text{ M SQUARE FOOTING} \cdot 1.0 \text{ M EMBEDMENT DEPTH.}$

CONCLUSIONES Y RECOMENDACIONES

De la investigación llevada a cabo en campo y los resultados obtenidos de las muestras recuperadas y realizando el análisis e interpretación de resultados, se llega a las siguientes conclusiones:

- El tipo de suelo encontrado en los sitios de los muestreos llevados a cabo, revelan una homogeneidad. Superficialmente se encuentra un material cohesivo con mucha grava fina arena desde los 50 centímetros de profundidad por el terreno natural. Aunque durante los trabajos de campo se detectó el nivel freático, se debe tener en cuenta que este puede variar de acuerdo con las estaciones del año y otros factores locales.
- Debido al tipo de suelo encontrado y la estratigrafía de los sondajes realizados se sugiere cimentar ampliando cimentaciones superficiales y apoyándose sobre el estrato firme o competente encontrado a partir de 1.00 metros de profundidad.
- Partiendo de las ecuaciones indicadas y las consideraciones tomadas en la Sección. 6, se estimó la capacidad portante para cimentaciones superficiales. Se sugiere cimentar a una profundidad mínima de 1.00 metros por debajo del terreno natural considerando a partir de ese punto una capacidad admisible resultó de suelo de 1.82 kgf/cm², para edificaciones considerarse un módulo de Winkler de 3.50 kgf/cm².
- En caso de proyectarse una cimentación de otra forma y/o dimension, se deben de considerar las dimensiones del área efectiva. De esta manera es posible aumentar el área de contacto de la cimentación con el suelo de tal forma que se logre transmitir las solicitaciones de la estructura. El ingeniero estructural, luego de analizar todos los factores que pueden afectar la estabilidad de la estructura, podrá determinar las dimensiones adecuadas para los rangos aplicados.

De acuerdo con la clasificación atómica correspondiente, es aplicable para la zona de estudio una modificación por el suelo de 0.30g. Adicionalmente se determinó que el tipo de suelo para aplicaciones sísmicas es S₂.

ESTUDIO DE SUELOS — GASOLINERA PUMA — JESÚS DE OTORO

26

CONCLUSIONS AND RECOMMENDATIONS

SHALLOW FOUNDATION ON A COMPETENT STRATUM AT 1.00 M - AVERAGE ALLOWABLE BEARING PRESSURE 1.89 KGF/CM² · PEAK GROUND ACCELERATION 0.30G, SOIL TYPE S₂.

PROJECT DETAILS

Location: Jesús de Otoro, Departamento de Intibucá, Honduras.

Period: 2024.

Area: 6118 m².

Role: Engineer of record · project's geotechnical basis (soil study) — CICH 8222.

SCOPE

The largest project in the portfolio by area — 6,118 m².

The scope documented here is the **geotechnical basis**: a 49-page soil report by ASP Consultores (laboratory, Tegucigalpa) — 3 PCA borings, grain-size classification, bearing capacity by Terzaghi's theory, and a design-parameter table. Does not include the station's complete structural design.

View project on [consuconstruct.com](https://consuconstruct.com/es/projects/gasolinera-puma-otoro) → (<https://consuconstruct.com/es/projects/gasolinera-puma-otoro>)

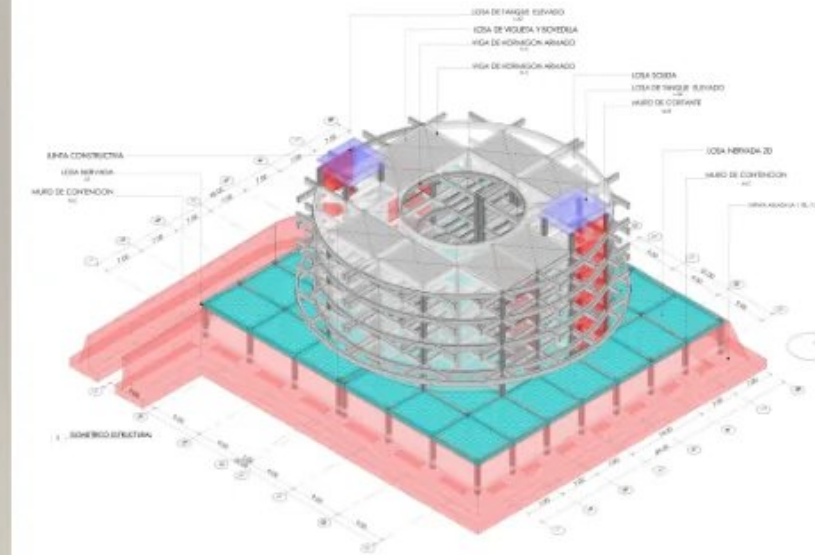
PROJECT DETAILS

Role: Structural design · technical documentation — CICH 8222.

SCOPE

Mixed-use building: retail on level 2 (+4.50 m) and offices on level 3 (+9.00 m), over a basement with isolated footing ZA-1 set at -7.03 m. C-1 column 0.70×0.70 m with #11 longitudinal steel and #4 stirrups — confinement zones at 0.10 m near the joints and 0.35 m at midspan, on a 0.80×0.80 m pedestal. V-1 load-bearing beam and V-2 tie beam, 0.45×0.90 m.

The system combines a **shear wall** and a **retaining wall** with three floor systems resolved separately: 2D ribbed slab, solid slab and joist-and-block slab, plus an elevated tank slab. The lightweight-slab details document the two alternatives that were costed — metal joist with galvadeck decking and 0.08 m topping, versus inverted T joist with concrete block filler.



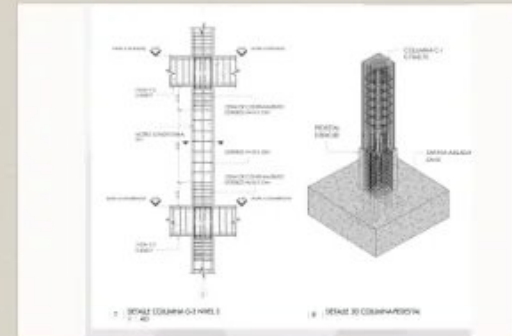
STRUCTURAL ISOMETRIC

SYSTEMS LABELED ELEMENT BY ELEMENT: 2D RIBBED SLAB · SOLID SLAB · JOIST-AND-BLOCK SLAB · ELEVATED TANK SLAB · SHEAR WALL · RETAINING WALL · CONSTRUCTION JOINT · ISOLATED FOOTING ZA-1 AT -7.03 M.



LIGHTWEIGHT SLABS — TWO ALTERNATIVES

METAL JOIST @ 1.7 M WITH GALVADECK DECKING AND E=0.08 M TOPPING · INVERTED T JOIST WITH CONCRETE BLOCK FILLER AND CONSTRUCTION JOINT.



COLUMN G-3 · LEVEL 2 (1:40) AND 3D COLUMN-PEDESTAL DETAIL

C-1 0.70×0.70 WITH #11 LONGITUDINAL STEEL · CONFINEMENT STIRRUPS #4 @ 0.10 M, MIDSPAN @ 0.35 M · PEDESTAL 0.80×0.80 · ISOLATED FOOTING ZA-01.



3D REINFORCEMENT DETAIL — JOINT

COLUMN C-1 WITH LOAD-BEARING BEAM V-1 AND TIE BEAM V-2, REINFORCEMENT AND LAP SPLICES SHOWN IN THREE DIMENSIONS.

COMMERCIAL AND OFFICE BUILDING

REINFORCED CONCRETE
STRUCTURE — MIXED USE

THE DATA STACK

07 WHY A DATABASE IN AN ENGINEERING PORTFOLIO

A price catalog is not a spreadsheet: it's a database with integrity constraints. Mine runs on **PostgreSQL 16** — 68 tables across 7 schemas — and carries the CSI catalog of **375 line items**, the **367 productivity-rated resources**, and the **7,344 resource breakdowns** that turn a quantity take-off into an auditable unit price.

pgvector adds semantic search inside the same SQL: vector(768) columns with cosine ivfflat indexes. In practice that means a description written in plain language — "30×30 pedestal with J-bolts" — finds its CSI code by similarity, not by text match. Without it, every new line item gets classified by hand.

What it solves on site: the same engine takes the Revit schedule, cross-references it against the catalog, assembles the budget, derives the schedule from each line item's productivity, and issues the PDF the bank accepts to the cent.

68 tables · 7 schemas · 7,749 vectorized chunks over 4,231 documents · 4 ivfflat indexes in production · local Nomic 768d embeddings, no external provider in the critical path.

07C BIM → BUDGET

49%

195 OF 398 LINE ITEMS WITH QUANTITY TAKEN
DIRECTLY FROM THE REVIT MODEL

The whole system rests on a single concept: **the CSI code**. Every element in the Revit model is born with its keynote embedded, and that same code travels the full chain — which is why the sheets in this portfolio carry the CSI legend printed next to the drawing. It's not decoration: it's the thread that lets the schedule export, cross-reference against each element's keynote, and land the quantity in the budget without anyone typing it in (field `revit_q`). What is normally days of quantity take-off is one pipeline run. A drawing that knows what it costs to build what it draws.

FIGURES AND TOOLS

07B PROJECT FIGURES (ACTUAL BUDGET)

PROJECT	CLIENT	LINE ITEMS	TOTAL L	LABOR L	MATERIALS L
Gasolinera Puma Otoro	Puma Energy	~26	24,921,068.15	—	—
CC-135 Fittacori	FITTACORI	~133	5,131,915.40	—	—
UNNEVENNESS (Santa Lucia)	Daniel Espinal	~343	2,972,776.53	—	—
Apt. Trapiche	Angela Guifarro	~193	1,655,987.42	—	—

Gasolinera Puma Otoro: preliminary architectural budget per source document, subject to adjustment for Puma Energy's own investment in materials and distribution equipment. UNNEVENNESS: two budget versions on file — figure shown is "Modelo Full"; reduced version = L 2,867,471.55. Labor/materials split not itemized in the source documents for these four engagements. Figures in Lempiras.

08 TOOLS BUILT

TOOL	WHAT IT IS	VERIFIED SCALE
EstimaStruct	In-house construction ERP	1,145 line items · 7,344 resource breakdowns · 367 resources · 90 CSI chapters · catalog v1.3, 375 line items
Structural engine	ACI 318-19 + AISC LRFD + CHOC-08 in Python	22/22 tests · Mathcad-style sheet in KaTeX · clause cited per step
ETABS 22 bridge	In-house C# stdio connector	Attaches to a live instance, MCP layer validated in production
Revit MCP	In-house server, public fork DavidChin0/revit-mcp-server	48 tools · 23 module families · Revit 2024–2027
pyRevit extensions	In production	Generate Layout · Conduit by Circuit · TypeMark TagAll
Brain	LangGraph orchestrator with human approval	299-entry tool registry · checkpoints in Postgres

09 BIM

Revit 2024–2027 at author/developer level, not operator.
2,000+ hours · 1,147 models · 66.7 GB across 80+ projects.
Villareal alone: 64 RVT files (architecture + dedicated plumbing + dedicated electrical + coordination model).
CSI keynote audit over 494 elements: dump → normalization → audit → approval → color sync.

04 SITE ARCHIVE

19,898 files · 290 GB · 889 dated folders · 45 months documented

12,865 site photographs · **7,007 site videos**

11 projects with their own folder linked to the public site

Continuous photo and video record 2019–2026. Villareal alone: 379 videos, 16.8 GB.

10 CONTRACT CHANNELS

Banks: Davivienda · Banpais · BAHPROVI · Atlántida · Lafise · Ficensa — budget PDF that matches the approved loan to the cent.

Institutional: Municipalidad de El Progreso · Registered UNDP supplier · Ciudad Mujer Presidential Project · FITTACORI.

Public works: active HonduCompras / ONCAE / CIRCE chain.

CONSUCONSTRUCT

STRUCTURAL · PORTFOLIO

david.chinchilla@consuconstruct.com

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